# Superior Wals Buid On A Superior Foundation" 

## How the Crushed Stone Footing Works

The physics of the crushed stone footing:

1. The purpose of any wall footing is to distribute the wall's load over a su $\square$ cient area of soil so that the weight-bearing capacity of the soil is not exceeded.
2. The load of the building is carried by the Superior Walls panel and is transferred to the $1 / 2$ " clean crushed stone.
3. The load distribution path through the crushed stone is at an angle approximately 60 degrees from the horizontal.
4. As the depth of the crushed stone layer increases, the e $\sqsubset$ ective bearing width on the underlying soil also increases. (See Figure 1.)
5. The tables in this booklet identify the required depth of the crushed stone footing for various wall loads and soil bearing capacities.


Figure 1

## Site Preparation

## Soils Veri cation

1. Determine your soil type from Table 1 on this page and stone depth requirements from Table 2 on page 6 . Superior Walls panels may be used on virtually any type of soil that has a bearing capacity of 1,500 PSF or better. For assistance identifying your soil type consult with:

- Building Department
- County Agricultural Extension Service
- County Conservation District O $\square$ cer
- Soils Technician
- Web Soil Survey website (http://websoilsurvey.nrcs.usda.gov)
- Excavator

2. Determine allowable Load-Bearing Pressure and Drainage Characteristics. (See Table 1.) This will a ect the required depth of the $1 / 2^{\prime \prime}$ clean crushed stone footing.
3. Establish combined footing load per linear foot. (Consider dead load, live load, snow and wind load.) Acquire loading information from building designer or engineer.
4. Determine required depth of the $1 / 2^{\prime \prime}$ clean crushed stone footing. (From Table 2. Remember to allow for this depth when determining excavation depth.)

| $\qquad$ <br> Table 1 <br> Properties of Soils Classi $\sqsubset$ ed According to the Uni $\square$ ed Soil Classi $\square$ cation System |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Table reference: 2018 IRC Table R405.1 |  |  |  |  |  |  |  |
| Soil Group | Unii ed Soil Classil cation System |  | Soil Description | Drainage Characteristics <br> (a) | Frost Heave Potential | Volume <br> Change <br> Potential <br> Expansion (b) | Presumptive Load-Bearing Pressure (PSF) (d) |
|  | Soil Classes | Lateral Soil Load (PCF) (f) |  |  |  |  |  |
| Group I Excellent | GW | 30 | Well graded gravel, gravel-sand mixtures, little or no $\square$ nes | Good | Low | Low | 3000 |
|  | GP | 30 | Poorly graded gravels or gravel sand mixtures, little or no ${ }^{\text {nes }}$ | Good | Low | Low | 3000 |
|  | SW | 30 | Well-graded sands, gravelly sands, little or no $\square$ nes | Good | Low | Low | 2000 |
|  | SP | 30 | Poorly graded sands or gravelly sands, little or no $\square$ nes | Good | Low | Low | 2000 |
|  | GM | 45 | Silty gravels, gravel-sand-silt mixtures | Good | Medium | Low | 2000 |
|  | SM | 45 | Silty sand, sand-silt mixtures | Good | Medium | Low | 2000 |
| Group IIFair to Good | GC | 45 | Clayey gravels, gravel-sand-clay mixtures | Medium | Medium | Low | 2000 |
|  | SC | 60 | Clayey sands, sand-clay mixture | Medium | Medium | Low | 2000 |
|  | ML | 45 | Inorganic silts and very $\square$ ne sands, rock $\rrbracket$ our, silty or clayey $\square$ ne sands or clayey silts with slight plasticity | Medium | High | Low | 1500(c) |
|  | CL | 60 | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays | Medium | Medium | Medium to Low | 1500(c) |
| Group III Poor (e) | CH | (e) | Inorganic clays of high plasticity, fat clays | Poor | Medium | High | 1500(c) |
|  | MH | (e) | Inorganic silts, micaceous or diatomaceous $\llbracket$ ne sandy or silty soils, elastic silts | Poor | High | High | 1500(c) |
| Group IV Unsatisfactory (e) | OL | (e) | Organic silts and organic silty clays of low plasticity | Poor | Medium | Medium | By Test |
|  | OH | (e) | Organic clays of medium to high plasticity, organic silts. | Unsatisfactory | Medium | High | By Test |
|  | PT | (e) | Peat and other highly organic soils | Unsatisfactory | Medium | High | By Test |

(a) The percolation rate for good drainage is over 4 inches per hour, medium drainage is 2 inches to 4 inches per hour, and poor is less than 2 inches per hour.
(b) Soils with a low potential expansion typically have a plasticity index (PI) of 0 to 15 , soils with a medium potential expansion have a Pl of 10 to 35 and soils with a high potential expansion have a PI greater than 20.
(c) Where the building ofi cial determines that in-place soils with an allowable bearing capacity of less than 1,500 psf are likely to be present at the site, the allowable bearing capacity shall be determined by a soils investigation. 2018 IRC Table R401.4.1.
(d) Presumptive Load-Bearing Values of Foundation Materials data from 2018 IRC Table R401.4.1.
(e) $\mathrm{CH}, \mathrm{MH}, \mathrm{OL}, \mathrm{OH}$, and PT are unsuitable as back 1 l material.
(f) Lateral soil load (PCF) from 2018 IRC tables in section R404.1

| Table 2 <br> Minimum Depth of 1/2" Clean Crushed Stone Footing (Inches) |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Construction Type |  | Soil Type \& Load Bearing Capacity (PSF) |  |  |  |  |  |
| Number of Stories | (Assumed Uniform Wall Load) ${ }^{(f)}$ | 1500 | 2000 | 2500 | 3000 | 3500 | 4000 |
|  |  | $\mathrm{MH}, \mathrm{CH} \text {, }$ CL, ML | $\begin{aligned} & \text { SC, GC, } \\ & \text { SM, GM, } \\ & \text { SP, SW } \end{aligned}$ |  | GP, GW |  |  |
| Conventional light-frame construction |  |  |  |  |  |  |  |
| 1 - Story | (1100 pounds per linear foot) ${ }^{(e)}$ | 4" | 4" | 4" | 4" | 4" | 4" |
| 2 - Story | (1800 pounds per linear foot) ${ }^{(\mathrm{e})}$ | 7" | 4" | 4" | 4" | 4" | 4" |
| 3 - Story | $\left(2900\right.$ pounds per linear foot) ${ }^{(e)}$ | $14^{\prime \prime}{ }^{(a)}$ | 9" ${ }^{(a)}$ | $5 "$ | 4" | 4" | 4" |
| Masonry veneer over light-frame construction |  |  |  |  |  |  |  |
| 1 - Story | (1500 pounds per linear foot) ${ }^{(e)}$ | 5" | 4" | 4" | 4" | 4" | 4" |
| 2 - Story | $\left(2700\right.$ pounds per linear foot) ${ }^{(e)}$ | $13^{\prime \prime}$ (a) | 8" | 4" | 4" | 4" | 4" |
| 3 - Story | (4000 pounds per linear foot) ${ }^{(\mathrm{e})}$ | 22" (a) | 14" (a) | 10" (a) | 7" | $5 "$ | 4" |
| (a) Crushed stone must be consolidated in 8 " lifts with a plate vibrator. <br> (b) Table allows for 378 pounds per linear foot for self weight of 10 ' Xi Plus foundation wall and 10-1/4 in. wall width. <br> (c) See Page 9 for Stone Speci cations. <br> ${ }^{(d)}$ Consult your Superior Walls drawing for the required depth of the crushed stone footing for your project. <br> (e) Assumed Wall Loading (pounds per linear foot) per 2018 IRC Table R403.4. <br> (f) The Assumed Uniform Wall Load shown in pounds per linear foot (plf) is the assumed load of the structure that is supported by the foundation wall and does not include the weight of foundation wall. |  |  |  |  |  |  |  |

## Typical Crushed Stone Footing Detail



Figure 2

